# Power Tower Model Structure Design and Numerical Analysis

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**Abstract:** Based on the model equivalence principle, the structural model of power pylons is designed and fabricated. The internal structure of power pylons is analyzed in terms of strength and stiffness under all levels of loading, and the structural stress and Z-directional displacement shapes are simulated and analyzed under typical operating conditions to verify the reasonableness of the model and provide reference for the subsequent design optimization of power pylon structures.

Keywords: Power Pylons, Model Design, ANSYS, Typical Working Conditions, Stress, Z-Directional Displacement

#### 1. Introduction

In this paper, the authors unite theory and practice, apply theoretical knowledge learned in the main courses of the profession and experience accumulated in the workplace to the design and production of the model, and build a link between real engineering and theoretical knowledge. In the whole stage of model design and production, not only the basic theoretical knowledge of mechanics and structure is applied, but also the practical experience of work and production is applied, which integrates the cultivation of hands-on practical skills and builds a bridge between practice and theory, while the quality of model production is closely related to practical hands-on ability. In turn, it has an unparalleled effect on the author's thinking and innovation, active learning, practical hands-on and comprehensive skill enhancement.

Power pylons are one of the important structural forms of high-rise building structures. How to design and fabricate power pylons and simulate the actual load condition for loading test as required becomes the key and difficult content. In this paper, we will introduce the ideas, structural selection and design, component fabrication and ANSYS numerical analysis.

#### 2. Introduction to the Idea

#### 2.1. Overview of Ideas

Design and make a model of a power tower, the bottom of the model using screws firmly placed in the size of  $400 \text{mm} \times 400 \text{mm} \times 15 \text{mm}$  (length×width×thickness) of the board, the size of the bottom of the model made strictly controlled in the center of the board 250 mm×250 mm square range, as shown in Figure 1 a), and the center of the board for the O point [1].

The model is equipped with 2 "low hanging points" and 1 "high hanging point" for subsequent loading to hang the wire, and the "high hanging point" is also used as a "horizontal The "high hanging point" is also used as a "horizontal loading point". The farthest outreach point is called the low hanging point, and the height between the lower base plate surface is limited to  $1000 \sim 1100$  (mm), and the projection formed on the lower base plate surface is in the circular shadow of the upper and lower sectors shown in Figure 1 a); the high hanging point is limited to  $1200 \sim 1400$  (mm), and the height between the lower base plate surface is limited to  $1200 \sim 1400$  (mm), and the height between the lower base plate surface is limited to  $1200 \sim 1400$  (mm), and the height between the lower base plate surface is limited to  $1200 \sim 1400$  (mm), and the height between the lower base plate surface is limited to  $1200 \sim 1400$  (mm), and the projection formed on the lower board surface is not far from the center O Projection, not more than 350mm from the center O point, and the model made by the high point is called high hanging point. The vertical position of the low and high hanging points of the model is shown in Figure 1b) [2].



Figure 1: Geometric range conditions of power tower model (mm).

#### 2.2. Material Properties

The materials used for the experiments were bamboo veneer paper and bamboo strips, whose bamboo specifications and material properties are referred to Table 1 and 2 below.

Туре	Specification (length×width×thickness mm)
Bamboo Veneer	1250×430×0.2 (single layer) 1250×430×0.35 (double layer) 1250×430×0.5
Paper	(double layer)
Bamboo strips	900×2×2 900×3×3 900×6×1

Table 1	: E	Bamboo	material	speci	fications.
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Table 2:	Bamboo	reference	mechanical	index.
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Density	0.8g/cm3	Compressive strength	30MPa
Parallel grain tensile strength	60MPa	Modulus of elasticity	6GPa

#### 3. Structure Selection

The choice of structural form of power tower should be based on the actual project span size, support conditions, plane style and other factors to implement the overall analysis to determine. Combined with the study and work experience, this experiment chose the lattice structure form.

#### 3.1. Component Analysis

Compared with the performance of bamboo veneer, bamboo strips have a better uniformity and the ultimate load increases with the increase of specimen size. Bamboo strips have defect zones (nodes), and when the length of the specimen is lengthened, the size of the material is also increasing, and the chance of the material defect zones appearing at this time is also increasing [3]. Therefore, when the length of the structural specimen is longer, its ultimate load value will be lower.

The model formed moments during the stage of applying tensile load, which weakened the ability of the model to bear the load due to the relatively large torque effect on the overall structure, meanwhile, the modulus of elasticity of the bamboo strip was relatively large and ductile, which caused the eccentric moments to have a more significant effect on the ultimate bearing capacity of the bamboo strip [4]. The test results show that the smaller the specimen size is, the more significant the effect of eccentric moment on the ultimate bearing capacity is, and the larger the specimen size is, the weaker the effect of eccentric moment on the ultimate bearing capacity is, but the specimen is too long, which will affect the stability of the compression bar, so the specimen length in the final confirmation scheme is chosen moderate [5].

#### 3.2. Column Design

The columns mainly bear the combined effects of vertical load, torque and tension. Considering the stability of the triangular truss structure, a total of two column structures are designed on this basis.

In Fig. 2, a) the structural form of column is a space truss structure, using bamboo strips pasted into T-beam splicing, the process is simple and easy to make, the column is wide at the top and thin at the bottom, and the bottom is enlarged feet to enhance its stability, but the loading test found that the program is not enough torsional stiffness, deformation is larger, and the self-weight is larger [6]. b) The structural form of column II uses bamboo veneer to make hollow square column, with stiffening ribs at the node position. Ribs, four columns with the same spacing, between the column and the column using the tension bar to resist the torque, the test found that the method of lighter self-weight, and torsional stiffness is larger, can be good resistance to the level 2 load, once after a comparison, the final determination of the column structure form using the b) Figure design [7].



a) Column structure form one b) Column structure form II Figure 2: Model of column structure.

#### 3.3. Main Structure Design

As shown in Figure 2 a), the main body of the model is designed and made into a truss system by bamboo strips in Scheme 1, and the oblique support system is added on the surface of each side of the truss structure, so that each surface is divided into individual small triangular structures, thus strengthening its structural stability [8]. The overall truss structure diagonal rods can all convey shear force well, while the deformation is small and the strength of the material can be utilized to the maximum [9]. The spatial layout of the model of Scheme 1 is symmetrical, flexible and rigid, but the torsional stiffness of the structure is insufficient, and the structure suffers torsional damage at 10cm~30cm below the low hanging point under Class 2 load, leading to structural failure [10].

As shown in Fig. 2 b), Scheme 2 designed the main body of the model in the form of lattice structure, and the four limbs of the structure were made of bamboo veneer paper to form a hollow square column section, and the four limbs relied on the tie bar to form a whole, which could play the role of torsional resistance, and the test proved that the tie bar had a good effect of torsional resistance [11]. The low hanging point bar can change the angle with the change of working condition to achieve effective bearing of three levels of load. It is verified by test that the scheme can better cope with various working conditions [12].



a) Option 1 b) Option Figure 3: Model processing diagram.

## 4. Modeling

#### 4.1. Component Material Analysis and Section Selection

According to the knowledge of civil engineering materials we know very well along the wood along the wood grain direction its tensile and compressive strength is greater, along the horizontal grain direction is the smallest[13]. see Figure 4, relative to the shear strength of the smallest shear strength direction is along the grain direction, in contrast to this along the horizontal grain direction is the largest, if the use of staggered grain adjacent veneer, although it can make the strength of each direction are very average, but the quality may not reach So the panel uses a single layer of vertical grain bamboo veneer paper, with bamboo veneer pulling strips at certain intervals horizontally, so that the vertical and horizontal strength of the panel does not differ much[14]. Therefore, it should be cut along the direction of the grain as much as possible when making the model.



#### Figure 4: Bamboo veneer pattern.

According to the knowledge of material mechanics, the ideal cross-sectional style of the beam should be maximized without increasing the cross-sectional size. $W_Z/A$  Such a cross-section is both suitable and economical [15]. The ratios of several common types of cross-sections $W_Z/A$  are listed in Table 3. Table 3 shows that I-beam and slotted sections are more economical than rectangular sections, while rectangular sections are more economical than circular sections [16].



Table 3:  $W_Z/A$  Values.

Therefore, when designing the cross-section of the beams in the model, rectangular cross-sections are used for the four main columns that mainly bear the combined effects of vertical load, torque and tension, and the shear effects are ignored for the other support system members of the structure, and I-beam cross-sections are partially selected under the consideration of the effects of bending moments, and T-shaped cross-sections.

#### 4.2. Node Construction

Since the connection between the members of the model is made by glue bonding, in order to make the connection between the members more secure and the model structure lighter in weight and more stable, the nodes should have a large contact surface with each other and reduce the material consumption at the same time [17]. Therefore, after several experiments we model node connection using two forms of Figure 5. According to the basic knowledge of mechanics, so in the subsequent modeling, the nodes between members are connected by rigid joints.

#### 5. Loading and Finite Element Analysis



a) Node construction I

b) Node construction II



#### 5.1. Loading Overview

The loading facilities of the model are mainly composed of the downhill gantry, the bearing plate, the uphill gantry and the lateral loading frame, as shown in Figure 6. The downhill and uphill gantry have 2 "low hanging points" and 1 "high hanging point" respectively, and the wires are manually hung on the corresponding hanging points of the downhill gantry, the model and the uphill gantry before loading as required. The wires are hung on the corresponding hanging points of the downhill gantry, model and uphill gantry manually before loading, as shown in Figure 6.

The position of the uphill gantry is unchanged during the experiment, and the downhill gantry can be rotated horizontally around the o point, and the rotation angle is  $0^{\circ}$ ,  $15^{\circ}$ ,  $30^{\circ}$  and  $45^{\circ}$  for selection (Figure 6 takes the rotation of  $30^{\circ}$  as an example).

Before the experiment, the base plate was fixed on the bearing plate, and the wires, loading discs

and horizontal wires were hung on the model according to the loading conditions extracted, which was called the "no-load" stage, while three laser distance meters were placed on the bearing plate at the bottom of the loading disc in the middle of each wire to measure the distance from the bottom of the loading disc to the face of the bearing plate. The distance between the bottom of the loading plate and the surface of the bearing plate.



Figure 6: Schematic of wire suspension.

In the loading process, according to the provisions of Table 4, it is necessary to ensure that the clearance height from the bottom surface of the loading plate to the surface of the bearing plate in the middle part of each conductor shall not be less than its corresponding value, otherwise it will be judged that the geometry of the model does not match or the level of loading fails.

*Table 4: Clearance limits between the bottom surface of the loading plate and the surface of the bearing plate.* 

Wire number	1	2	3	4	5	6
Clearance limit (mm)	400	600	400	800	1000	800

And according to the experimental requirements, the wire loading conditions have the following four for selection, as shown in Table 5.

Number Work conditions	Lead 1	Lead 2	Lead 3	Lead 4	Lead 5	Lead r 6
А	$\checkmark$	$\checkmark$				$\checkmark$
В	$\checkmark$				$\checkmark$	$\checkmark$
С		$\checkmark$	$\checkmark$	$\checkmark$		
D			$\checkmark$	$\checkmark$	$\checkmark$	

Table 5: Lead wire loading conditions.

#### 5.1.1. Overview of Loading at All Levels

One-stage loading: (1) Pick one of the 3 loading wires and give weights on the loading disc of its picked wire, then the total weight of the applied weights at this stage is counted as M1, as shown in Fig. 7 a) (with the working condition A rotated by  $30^{\circ}$  for example, the same below). (2) When the loading is finished, it needs to stay for 10s, and then it needs to obtain the headroom height information of the 3 laser rangefinders under the loading disc, if the headroom height information is less than the requirement of Table 4 or the display is abnormal, the loading will be regarded as a failure.

Secondary loading: (1) The first-level load applied in the previous part will be held, while weights are applied on the loading disc for the remaining 2 specified wires, then the total weight of the applied weights at this level is counted as M2, as shown in Figure 7b). (2) When the loading is finished, it needs to stay for 10s, and then the headroom values of the 3 laser distance meters under the loading disc need to be obtained, and if the values are less than the requirements of Table 4 or show abnormalities, the loading will be judged to have failed.

Three levels of loading: (1) the first and second levels of loading applied in the previous part remain

unchanged. At the same time, the lateral horizontal load is given at the location of the "horizontal loading point" of the model using the "weight + guide rope" requirement, and the size of the applied load weight can be chosen between 4kg and 10kg, which is M3, as shown in Figure 7c). (2) When the loading is finished, it is necessary to stay for 10s, after which if the model overturns or collapses as a whole, or if the wire falls or is unbooked, the loading will be judged to have failed.



Figure 7: Loading schematic.

## 5.2. Finite Element Analysis

According to the experimental requirements, the four loading conditions and the rotation angles of  $0^{\circ}$ ,  $15^{\circ}$ ,  $30^{\circ}$  and  $45^{\circ}$  were loaded respectively in Table 5. According to the structural model, the structure was simulated by the finite element ANSYS software, and the rods were simulated by the built-in BEAM188 unit of ANSYS software, and the material parameters were taken according to Table 2[6]. Through the static analysis, the finite element model and the structure equivalent force cloud and Z to displacement cloud two results were extracted, respectively, from the strength and stiffness of the structure in the loading process of the two aspects of the force under all levels of load. The tensile strength of bamboo is 30 MPa, and the structure may be damaged once the equivalent force exceeds the theoretical ultimate tensile stress.

#### 5.2.1. Finite Element Model

According to the above model appearance size requirements, loading requirements, we take the actual model for example; Figure 8 shows the complete finite element model.



Figure 8: Finite element model.

#### 5.2.2. Analysis of Working Conditions

According to the experimental requirements of, the four loading conditions and rotation angles of  $0^\circ$ ,  $15^\circ$ ,  $30^\circ$  and  $45^\circ$  were used in Table 4-2, and the loads were loaded in accordance with the full load of 4kg in the first level, 3kg in the second level and 10kg in the third level. For simplicity, this section only shows the finite element analysis of the four loading conditions with the rotation angle of  $30^\circ$ , and outputs the displacement cloud in Z direction and the equivalent force cloud, and gives the model optimization suggestions for the analysis results.



*a)* Working condition *A b)* Working condition *B c)* Working condition *C d)* Working condition *D* 

Figure 9: Loading diagram with a rotation angle of 30°.

(1) Condition A: The loading method is 4kg for primary load, 3kg for secondary load, and 10kg for tertiary load, and the results of ANSYS simulation are shown in Figure 10 and Figure 11. According to the calculation, the maximum equivalent force of the overall structure is 54.7MPa, and the tensile strength of bamboo veneer paper is 60MPa, which meets the requirements, the maximum equivalent force is located at the foot of the column, so the foot of the column needs to be focused on. According to the test measurement, the sinkable limit is 115mm. According to the finite element calculation result, the Z-directional displacement deformation value is 4.9mm, which meets the requirement.



Figure: 10 Z-direction displacement cloud.

Figure 11: Equivalent force cloud.

(2) Condition B: The loading method is 4kg for primary load, 3kg for secondary load, and 10kg for tertiary load, and the results of ANSYS simulation are shown in Figure 12 and Figure 13. According to the calculation, the maximum equivalent force of the overall structure is 52.6MPa, and the tensile strength of bamboo veneer paper is 60MPa, which meets the requirements, the maximum equivalent force is located at the foot of the column, so the foot of the column needs to be focused on. According to the test measurement, the sinkable limit is 115mm. According to the finite element calculation result, the Z-directional displacement deformation value is 12.4mm, which meets the requirement.



*Figure 12: Z-direction displacement cloud. Figure 13: Equivalent force cloud.* 

(3) Condition C: The loading method is 4kg for primary load, 3kg for secondary load, and 10kg for

tertiary load, and the results of ANSYS simulation are shown in Figure 14 and Figure 15. According to the calculation, the maximum equivalent force of the overall structure is 52.2MPa, and the tensile strength of bamboo veneer paper is 60MPa, which meets the requirements, the maximum equivalent force is located at the foot of the column, so the foot of the column needs to be focused on. According to the test measurement, the sinkable limit is 115mm. According to the finite element calculation result, the Z-directional displacement deformation value is 3.9mm, which meets the requirement.





Figure 14: Z-direction displacement cloud.

Figure 15: Equivalent force cloud.

(4) Condition D: The loading method is 4kg for primary load, 3kg for secondary load, and 10kg for tertiary load, and the results of ANSYS simulation are shown in Figure 16 and Figure 17. According to the calculation, the maximum equivalent force of the overall structure is 52.7MPa, and the tensile strength of bamboo veneer paper is 60MPa, which meets the requirements, the maximum equivalent force is located at the foot of the column, so the foot of the column needs to be focused on. According to the test measurement, the sinkable limit is 115mm. According to the finite element calculation result, the Z-directional displacement deformation value is 12.4mm, which meets the requirement.



Figure 16: Z-direction displacement cloud. Figure 17: Equivalent force cloud.

Due to space reasons this section only shows the model to rotate the angle of 30  $^{\circ}$  as an example of finite element analysis, other angles of the data analysis is shown in Table 6, from the table data can be seen that the model for the experimental various conditions of its equivalent pressure maximum and Z-directional displacement values are to meet the bamboo veneer paper smooth grain tensile strength of 60MPa, sinking displacement limit of 115mm, the model structure is reliable.

Table 6: Working condition analysis data sheet.

Angle	Work conditions	Equivalent pressure maximum	Z-directional displacement value		
	А	42.3MPa	6.25mm		
0.9	В	42.8MPa	13.5 mm		
0°	С	42.4MPa	6.05mm		
	D	42.8MPa	13.4mm		
	А	23.1MPa	9.84mm		
150	В	43.5MPa	11.7mm		
15-	С	48.5MPa	4.97mm		
	D	33.4MPa	11.4mm		
	Α	54.7MPa	4.9mm		
200	В	52.6MPa	12.4mm		
30-	С	52.2MPa	3.9mm		
	D	52.7MPa	12.4mm		
	Α	41.2MPa	2.56mm		
150	В	34.6MPa	12.2mm		
43	С	37.9MPa	3.34mm		
F	D	44.3MPa	12.3mm		

#### 6. Summary

This paper carries out the structural selection, modeling and finite element analysis of the power tower model, the angle of rotation of the model, different working conditions for the graded loading analysis of the strength and stiffness of the internal structure of the power tower, to obtain the corresponding stress and Z-directional displacement shape of the structure, pointing out the equivalent force maximum parts, and compared with the experimental limits of each working condition, to meet the relevant provisions, the model structure is reliable.

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